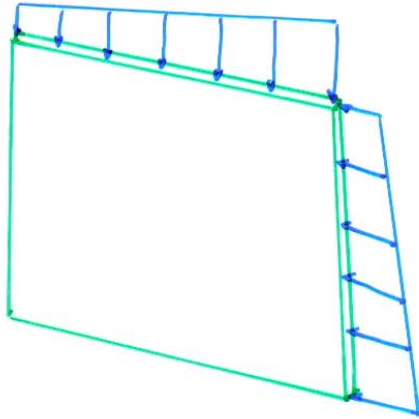


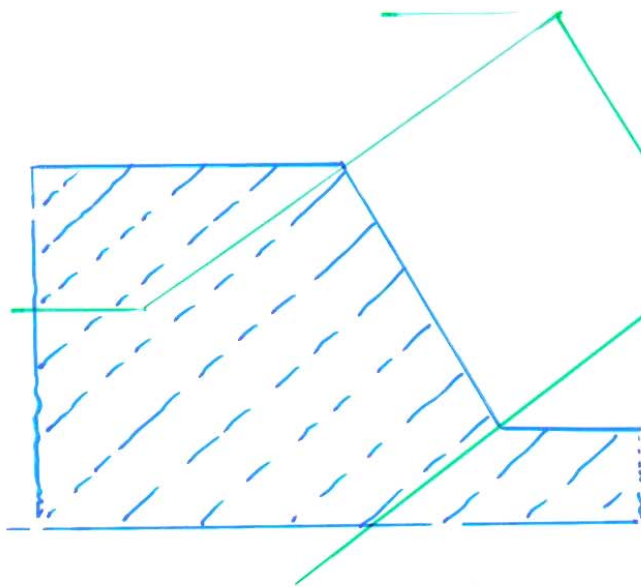
TEMA 6 - TENSIONES EN ROCA

TENSIONES EN EL PLANO



TENSION PLANA

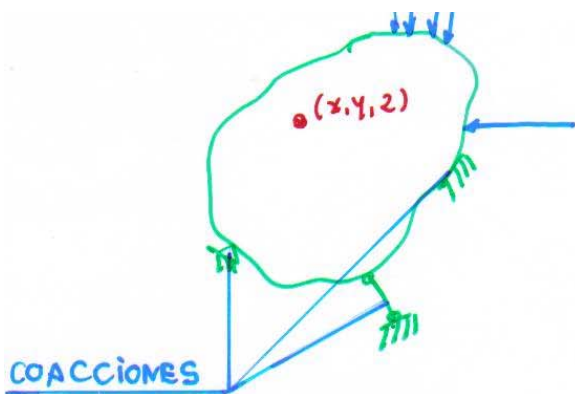
$$\sigma_2 = 0$$



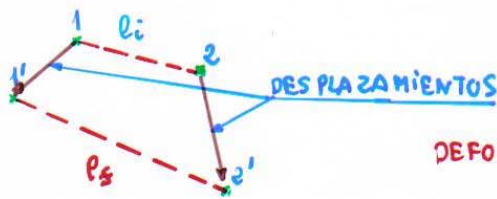
DEFORMACION PLANA

$$\sigma_2 = f(\sigma_1, \sigma_3)$$

$$\sigma_1 > \sigma_2 > \sigma_3$$



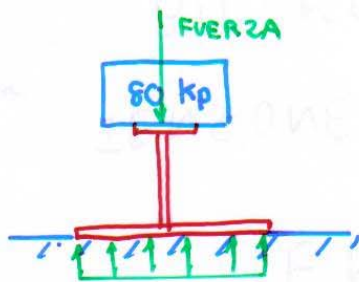
ESTADO MECANICO DEL SOLIDO



DEFORMACION $E = \lim \frac{l_f - l_i}{l_i}$

(ESCALAR)

si $2 \rightarrow 1 \rightarrow l_i \rightarrow 0$ $\vec{\epsilon}_{ij} = \frac{\partial d_i}{\partial x_j}$ VECTORIAL!



TENSION = FUERZA / AREA

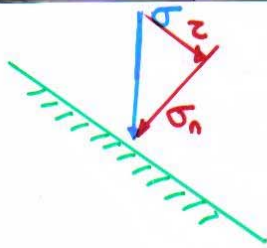
$$\vec{\sigma} = \lim_{A \rightarrow 0} \frac{\vec{F}}{A} = \frac{d\vec{F}}{dA}$$

\vec{F} : Vector A: escalar $\Rightarrow \vec{\sigma}$: VECTORIAL

CRITERIO DE SIGNOS



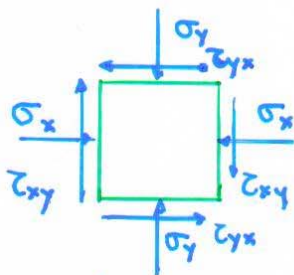
DESCOMPOSICIÓN DE TENSION



σ_n : TENSION NORMAL
 σ : TENSION TOTAL
 τ_{xy} : TENSION TANGENCIAL

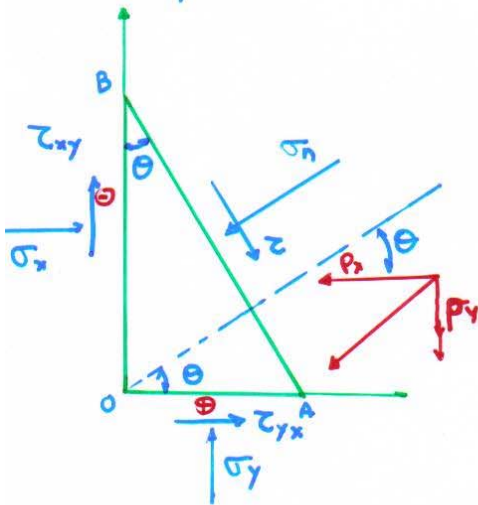
EL ESTADO DE TENSIONES EN UN PUNTO QUEDA DEFINIDO

POR LOS VALORES EN DOS PLANOS PERPENDICULARES



EQUILIBRIO MOMENTOS $\tau_{yx} = \tau_{xy}$

$\tau(x,y)$
 EJE AL QUE ES PARALELO
 PLANO "x" = Cte. EN QUE SE CONTIENE



ESTABLECIENDO EQUILIBRIO

$$P_x \cdot AB = \sigma_x \cdot OB + \tau_{yx} \cdot OA$$

$$OB = AB \cdot \cos \theta$$

$$OA = AB \cdot \sin \theta$$

↓

$$P_x = \sigma_x \cdot \cos \theta + \tau_{yx} \cdot \sin \theta$$

$$P_y = \sigma_y \cdot \sin \theta + \tau_{xy} \cdot \cos \theta$$

Si $\tau_{xy} = \tau_{yx} = \tau$

$$\sigma_x = \sigma_1$$

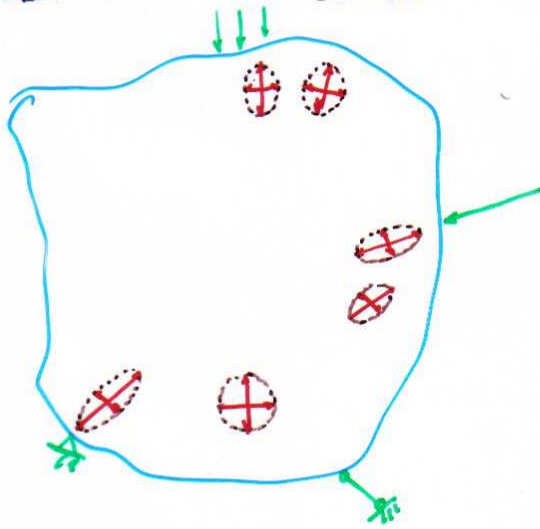
$$P_x = \sigma_1 \cdot \cos \theta$$

$$\sigma_y = \sigma_3$$

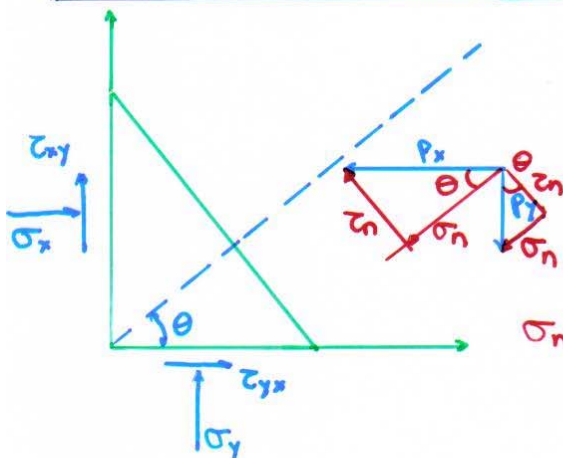
$$P_y = \sigma_3 \cdot \sin \theta$$

$$\frac{P_x^2}{\sigma_1^2} + \frac{P_y^2}{\sigma_3^2} = 1 \Rightarrow \text{ELIPSES DE TENSIONES}$$

LAS TENSIONES EN EL PLANO → ELIPSES DE TENSIONES



CIRCULO DE MOHR, ISOSTATICAS



$$\sigma_n = p_x \cos \theta + p_y \sin \theta$$

$$\tau = -p_x \sin \theta + p_y \cos \theta$$

$$\sigma_n = \sigma_x \cos^2 \theta + 2\tau_{xy} \sin \theta \cos \theta + \sigma_y \sin^2 \theta$$

↓

$$\sigma_n = \frac{1}{2} (\sigma_x + \sigma_y) + \cos 2\theta \frac{1}{2} (\sigma_x - \sigma_y) +$$

$$\tau_{xy} \sin 2\theta$$

$$\tau = (\sigma_y - \sigma_x) \sin \theta \cos \theta + \tau_{xy} (\cos^2 \theta - \sin^2 \theta)$$

$$\tau = \frac{1}{2} (\sigma_y - \sigma_x) \sin 2\theta + \tau_{xy} \cos 2\theta = \cos 2\theta \left[(\sigma_y - \sigma_x) \frac{1}{2} \times \right. \\ \left. \tan 2\theta + \tau_{xy} \right]$$

SI HACEMOS ~~τ=0~~ $\tau=0 \Rightarrow 0 = \frac{1}{2}(\sigma_y - \sigma_x) \cdot \text{Tg } 2\theta + \tau_{xy}$

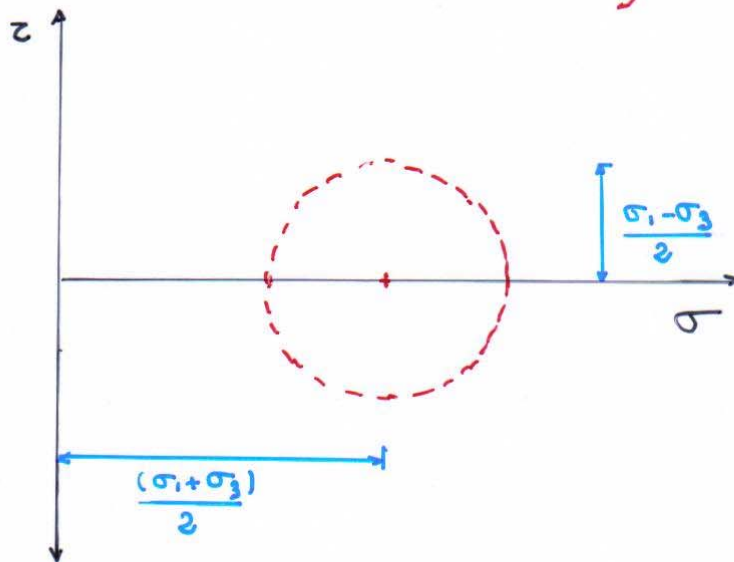
Ecuación ISOSTÁTICAS

SI HACEMOS $\sigma_y = \sigma_3$ $\sigma_x = \sigma_1$ $\tau_{xy} = 0$

$\sigma_n = \frac{1}{2}(\sigma_1 + \sigma_3) + \frac{1}{2}(\sigma_1 - \sigma_3) \cos 2\theta$

$\tau = \frac{1}{2}(\sigma_1 - \sigma_3) \sin 2\theta$

\Downarrow
 CIRCULO CENTRO : $\frac{1}{2}(\sigma_1 + \sigma_3), 0$
 RADIO $\frac{1}{2}(\sigma_1 - \sigma_3)$ } CIRCULO DE MOHR.



CIRCULO DE MOHR \leftrightarrow SISTEMA GRAFICO DE OBTENCION DE TENSIONES EN CUALQUIER DIRECCION DE CASO PLANO.

\downarrow
INDEPENDIENTE CRITERIO DE ROTURA

ISOSTÁTICAS

Se tenía $\tau = \cos 2\theta \cdot \left[\frac{1}{2} (\sigma_y - \sigma_x) \cdot \operatorname{tg} 2\theta + \tau_{xy} \right]$

HACIENDO $\tau = 0 \Rightarrow \operatorname{tg} 2\theta = \frac{2 \tau_{xy}}{\sigma_y - \sigma_x}$

$$\operatorname{Tg} 2\theta = \frac{2 \operatorname{tg} \theta}{1 - \operatorname{Tg}^2 \theta} = \frac{2 \frac{dy}{dx}}{1 - \left(\frac{dy}{dx}\right)^2} = \frac{2 \tau_{xy}}{\sigma_y - \sigma_x}$$



$$\left[\frac{dy}{dx} \right]_1^2 = -\frac{\sigma_x - \sigma_y}{2 \tau_{xy}} \pm \sqrt{1 + \frac{(\sigma_x - \sigma_y)^2}{(2 \tau_{xy})^2}}$$

ECUACION DIFERENCIAL DE LAS ISOSTÁTICAS

$$\left[\frac{dy}{dx} \right]_1 \times \left[\frac{dy}{dx} \right]_2^2 = -1 \Rightarrow \text{PERPENDICULARES}$$